Approved Building Consent Documents

Please Note: A copy of the stamped approved documents must be available on site for all inspections.

Inspection booking timeframes

Call received	before 3pm inspection will be done	after 3pm inspection will be done
Monday	Wednesday	Thursday
Tuesday	Thursday	Friday
Wednesday	Friday	Monday
Thursday	Monday	Tuesday
Friday	Tuesday	Wednesday

Building inspections and enquiries phone: 03 347 2839

Please ensure all work for inspection is ready the day before. Incomplete work requiring re-inspection will incur an additional inspection fee.

Reference Number: 180748.1

Project Location: 116 McDonald Road Lincoln

Project Description: Farm Building

Resource Consent # RC195342 has been granted on 03/07/19

3/07/2019 bigsbr

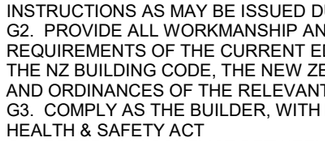
GENERAL

G1. READ THESE DRAWINGS IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER WORKING DRAWINGS, SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT
G2. PROVIDE ALL WORKMANSHIP AND MATERIALS IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITIONS OF THE NZ BUILDING ACT, THE NZ BUILDING CODE, THE NEW ZEALAND STANDARDS AND THE BY-LAWS AND ORDINANCES OF THE RELEVANT BUILDING CONSENT AUTHORITY.
G3. COMPLY AS THE BUILDER, WITH REQUIREMENTS OF THE WORKPLACE HEALTH & SAFETY ACT
G4. REFER ANY CONFLICT BETWEEN THESE NOTES, THE SPECIFICATION, THE DRAWINGS OR ANY OTHER RELEVANT DOCUMENTS TO THE ENGINEER FOR DECISION PRIOR TO PROCEEDING WITH THE WORK.
G5. DO NOT OBTAIN DIMENSIONS BY SCALING THE DRAWINGS. FOR SETTING OUT DIMENSIONS AND LEVELS REFER TO ARCHITECTURAL DRAWINGS.
G6. THE BUILDER IS RESPONSIBLE FOR THE PROVISION OF ALL SHORING TO MAINTAIN THE STABILITY AND INTEGRITY OF EXCAVATIONS AND ADJACENT STRUCTURES. PROVIDE DETAILS, FOR REVIEW BY THE ENGINEER, OF ANY NECESSARY TEMPORARY WORKS, INCLUDING SHORING, PRIOR TO COMMENCING CONSTRUCTION.
G7. DURING CONSTRUCTION IT IS THE BUILDER'S RESPONSIBILITY TO MAINTAIN THE STRUCTURE IN A STABLE CONDITION AND TO ENSURE NO PART IS OVERSTRESSED.
G8. THE DESIGN AND DRAWINGS ARE COPYRIGHT AND MAY NOT BE USED OR REPRODUCED IN WHOLE OR IN PART WITHOUT THE WRITTEN PERMISSION OF THE ENGINEER.
G9. FIRE-RESISTANT LEVELS (FRL'S) REQUIRED FOR THE VARIOUS STRUCTURAL ELEMENTS MUST BE CONFIRMED BY THE BUILDING SURVEYOR OR ARCHITECT.

FOUNDATIONS

F1. THE MINIMUM SAFE BEARING CAPACITY OF FOUNDATION MATERIAL SHALL BE:
SLABS & FOOTINGS : 300 kPa.(ULS) U.N.O.
F2. FOUNDATION MATERIAL SHALL BE APPROVED BY THE ENGINEER PRIOR TO PLACING CONCRETE.
F3. THE BASES OF FOOTING EXCAVATIONS SHALL BE FINISHED CLEAN AND HORIZONTAL AND BE EMBEDDED 300 INTO BEARING GROUND OR AS NOTED ON DETAILS.
F4. FOUNDING LEVELS WHERE SHOWN ARE FOR TENDER PURPOSES ONLY.
F5. ANY PROPOSED FOOTING EXCAVATION NEAR BOUNDARIES, OTHER STRUCTURES OR SERVICES SHALL BE APPROVED BY THE ENGINEER.
F6. SUBGRADE SHALL BE APPROVED MATERIAL COMPACTED TO 98% STANDARD DRY DENSITY DETERMINED BY TESTING TO THE RELEVANT PART OF NZS 4402: 1986 U.N.O
F7. LOCATE ALL NEW FOOTINGS RELATIVE TO LINE OF CUT/EXCAVATION INCLUDING EXCAVATIONS AS FOLLOWS:

LINE OF INFLUENCE



FOUND ALL FOUNDATIONS BELOW LINE OF INFLUENCE OF EXISTING CUT OR EXISTING FOOTINGS UNLESS NOTED

2 FOR SAND
1 FOR CLAY
0 FOR ROCK

*SUBJECT TO APPROVAL FROM THE GEOTECHNICAL ENGINEER

LOADINGS

L1. SUPERIMPOSED FLOOR LOADS ARE IN ACCORDANCE WITH AS/NZS1170.1:
DOMESTIC FLOORS : 1.5 kPA
BALCONIES : 2.0kPa
STAIRS : 2.0kPa

L2. WIND LOADS ARE IN ACCORDANCE WITH AS/NZS1170.2 FOR:
TERRAIN CATEGORY: 2.5 REGION A7
SHIELDING MULTIPLIER OF 1.0
TOPOGHRAPHIOC MULTIPLIER OF 1.0

L3. SNOW LOADS ARE IN ACCORDANCE WITH AS./NZS1170.3

L4. EARTHQUAKE LOADS ARE IN ACCORDANCE WITH NZS1170.5

FIGURED DIMENSIONS TAKE PRECEDENCE.

REINFORCED CONCRETE

C1. PROVIDE ALL WORKMANSHIP AND MATERIALS IN ACCORDANCE WITH NZS 3101.1:2006, AND NZS 3109:1997, THEIR REFERENCED DOCUMENTS, THE DRAWINGS AND THE SPECIFICATION.
C2. PROVIDE CONCRETE COMPOSITION AND MINIMUM CLEAR CONCRETE COVER TO REINFORCEMENT AS FOLLOWS:-

ELEMENT	F'c MPa	NZS 3604 COVER mm
SLABS & FOOTINGS	20	75 BTM & SIDES (WHERE NO D.P.M) 50 BTM & SIDES (WHERE D.P.M) 35 TOP

C3. SUPPORT ALL REINFORCEMENT AT 1 METRE MAXIMUM CENTRES BOTH WAYS ON MILD STEEL PLASTIC TIPPED CHAIRS, PLASTIC CHAIRS OR CONCRETE CHAIRS. USE ONLY PLASTIC CHAIRS FOR EXTERNALLY EXPOSED SOFFITS.
C4. PROVIDE ALL CONCRETE WITH 80mm MAXIMUM SLUMP, 20mm MAXIMUM AGGREGATE WITH NO ADMIXTURES, UNLESS APPROVED BY THE ENGINEER.
C5. SIZES OF CONCRETE ARE NET, EXCLUSIVE OF APPLIED FINISHES. BEAM DEPTHS ARE WRITTEN FIRST AND INCLUDE SLAB THICKNESS.
C6. PROPERLY FORM CONSTRUCTION JOINTS AND USE ONLY WHERE SHOWN ORAPPROVED BY THE ENGINEER.
C7. DELETED
C8. REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY AND IS NOTNECESSARILY SHOWN IN TRUE PROJECTION.
C9. WELD OR SPLICE REINFORCEMENT ONLY IN POSITIONS APPROVED BY THE ENGINEER.
C10. PROVIDE THE MINIMUM CLEAR SPACING BETWEEN CONDUITS, CABLES, PIPES AND BARS AS REQUIRED BY NZS 3109 BUT NOT LESS THAN THREE BARDIAMETERS. DO NOT PLACE CONDUITS IN SLABS ABOVE TOP REINFORCEMENT OR BELOW BOTTOM REINFORCEMENT.
C11.

D - DENOTES HOT ROLLED DEFORMED BARS GRADE 300E.
HD - DENOTES HOT ROLLED DEFORMED BARS GRADE 500E.
R - DENOTES HOT ROLLED PLAIN ROUND BARS GRADE 230R.
SE, RE - DENOTES HARD DRAWN WIRE FABRIC GRADE 500E.

C12. NOTIFY THE ENGINEER A MINIMUM OF 24 HOURS BEFORE REINFORCEMENT HAS BEEN COMPLETED. ALLOW 2 HOURS AFTER THE COMPLETION OF THE REINFORCEMENT FOR THE ENGINEER'S INSPECTION. DO NOT ORDER REINFORCEMENT FOR THE ENGINEER'S INSPECTION. DO NOT ORDER CONCRETE UNTIL REINFORCEMENT HAS BEEN APPROVED BY THE ENGINEER.
C13. CURE CONCRETE IN ACCORDANCE WITH NZS 3109. COMMENCE CURING WITHIN TWO HOURS OF FINISHING OPERATIONS AND CONTINUE FOR A MINIMUM OF SEVEN DAYS BY USING AN APPROVED PROPRIETARY COMPOUNDOR BY KEEPING CONTINUOUSLY WET.
C14. TIE ALL UNSUPPORTED BARS IN TRANSVERSE DIRECTION TO D12-300, LAPPED 500 U.N.O.
C15. LAP FABRIC IN ACCORDANCE WITH CLAUSE 8.7.6 OF NZS 3109.
C16 PROVIDE HOOKS, LAPS AND BENDS IN ACCORDANCE WITH NZS 3101 & NZS 3109 U.N.O.
C17. PROVIDE CHAMFERS, DRIP GROOVES ETC. IN ACCORDANCE WITH THE ARCHITECT'S DETAILS.
C18. DESIGN, CONSTRUCT AND STRIP FORMWORK IN ACCORDANCE WITH NZS 3109.
C19. PRE CAMBER FORMWORK UPWARDS BY 1/500 OF THE CLEAR SPAN U.N.O.WHERE SUPPORTED BEAMS AND SLABS SPAN GREATER THAN 5m.
C20. THE CONCRETE ELEMENTS DETAILED HAVE AN F.R.L. OF 60/60/60 U.N.O.

TIMBER

T1. ENSURE ALL WORKMANSHIP AND MATERIALS ARE IN ACCORDANCE WITH NZS 3603:1993 AND NZS 3604:2011, THEIR REFERENCED DOCUMENTS AND THE SPECIFICATION.
T2. PROVIDE ALL INTERIOR TIMBER AS SG6 (DRY) U.N.O.
PROVIDE ALL EXTERNAL TIMBER ASSG8 (DRY) U.N.O.
T3. WHERE THE USE OF TREATED TIMBER FOR DURABILITY IS NOTED ON THE STRUCTURAL DRAWINGS, ENSURE IT COMPLIES WITH THE PARAMETERS:
EXPOSURE ZONE C IN ACCORDANCE WITH NZS 3604
T4. INSTALL PROPRIETARY TIMBER CONNECTORS IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS.
T5. RETIGHTEN BOLTED CONNECTIONS IN UNSEASONED TIMBER PRIOR TO THE FIXING OF CLADDING.
T6. TIMBER ELEMENTS OR TIMBER FRAMING HAVE NOT BEEN DESIGNED UNLESS NOTED
T7. ALL TRUSSES TO BE PRE-CAMBERED UPWARD 1/240 SPAN U.N.O.

Revision Schedule

Revision Number	Revision Description	Revision Date

MASONRY

M1. ENSURE ALL WORKMANSHIP AND MATERIALS ARE IN ACCORDANCE WITH NZS 4230, THE DRAWINGS & STANDARD TECHNICAL SPECIFICATIONS.
M2. WHERE MASONRY SUPPORTS CONCRETE SLABS OR BEAMS, LAY THE TOP COURSE WITH FROGS DOWN AND COVERED WITH 2 LAYERS OF APPROVEDSLIP JOINT MATERIAL.
M3. WALLS SHOWN SHADED ON PLAN ARE LOAD BEARING.
SEPARATE NON-LOAD BEARING WALLS UNDER SLABS FROM THE SLAB BY 15mm OF APPROVED COMPRESSIBLE MATERIAL.
WHERE MASONRY ABUTS SLAB DOWNTURNS,15mm GAP BETWEEN BRICKWORK AND SIDE OF DOWNTURN.
M4. DO NOT ERECT MASONRY SUPPORTED BY CONCRETE SLABS OR BEAMS UNTIL ALL FORMWORK AND PROPS UNDER HAVE BEEN REMOVED.
M5. PROVIDE ALL BRICKS OF STRENGTH F'UC= 20 MPa U.N.O.
M6. PROVIDE ALL HOLLOW CONCRETE MASONRY OF STRENGTH F'UC = 15 MPa U.N.O.
M7. PROVIDE CLASSIFICATION M3 MASONRY MORTAR U.N.O. NOTE THAT WITHIN100m FROM NON-SURF COAST, OR 1KM FROM SURF COAST, PROVIDE CLASSIFICATION M4 MASONRY MORTAR.
M8. CUT NO CHASES INTO LOADBEARING MASONRY WITHOUT THE APPROVAL OFTHE ENGINEER.
M9. PROVIDE MOVEMENT CONTROL JOINTS VERTICALLY FOR FULL HEIGHT OFWALL AS FOLLOWS:
FOR GENERAL MASONRY, AT 8m MAXIMUM CENTRES & 4m MAXIMUM FROM CORNERS.
FOR ARTICULATED MASONRY, AT 6m MAXIMUM CENTRES & 4m MAXIMUM FROM CORNERS.
PROVIDE 15mm MINIMUM JOINTS WITH AN APPROVED COMPRESSIBLEFILLER, TIED TOGETHER EVERY 4TH COURSE WITH AN MET 3.3 MASONRY SLIDING TIE OR APPROVED EQUAL.
M10. CONSTRUCT HOLLOW WALLS TO FULL HEIGHT OR MAXIMUM 3m BEFORE FILLING CORES.
PROVIDE CLEANOUT OPENINGS AT THE BASE OF ALL CORES TO BE FILLED.
M11. PROVIDE HOLLOW F'C 20 MPa CORE FILLING CONCRETE WITH 10mm AGGREGATE, 180 SLUMP U.N.O.
M12. CONSTRUCT HOLLOW MASONRY RETAINING WALLS USING "DOUBLE U BLOCKS".
M13. UNREINFORCED MASONRY WALLS HAVE NOT BEEN DESIGNED UNLESS NOTED.
M14. REINFORCED MASONRY WALLS HAVE AN F.R.L. OF -/-/- U.N.O.

STEELWORK

S1. ENSURE MATERIALS, FABRICATION AND ERECTION ARE IN ACCORDANCEWITH NZS 3404, ITS REFERENCED DOCUMENTS AND THE SPECIFICATION.
S2. SUBMIT THREE COPIES OF ALL WORKSHOP DRAWINGS TO THE ARCHITECTAND THE ENGINEER TO OBTAIN THEIR WRITTEN APPROVAL PRIOR TO FABRICATION.
S3. PROVIDE ALL WELDS AS 6mm CONTINUOUS FILLET FROM E41XX ELECTRODES, ALL BOLTS AS M16-4.6/S AND ALL CLEATS AND GUSSETS AS10mm PLATE U.N.O.
S4. FOR BOLTS, THE FOLLOWING NOTATION IS USED:
4M16-4.6/S DENOTES 4 X M16 COMMERCIAL GRADE BOLTS SNUG TIGHT.
6M20-8.8/TF DENOTES 6 X M20 HIGH STRENGTH STRUCTURAL BOLTS FULLY TENSIONED IN A NO SLIP JOINT.
8M24-8.8/TB DENOTES 8 X M24 HIGH STRENGTH STRUCTURAL BOLTS FULLY TENSIONED IN A BEARING JOINT.
S5. LEAVE MATING SURFACES OF TF CONNECTIONS UNPAINTED AND FREE OF MILL SCALE AND RUST.
S6. TIGHTEN BOLTS IN TF AND TB CONNECTIONS USING THE PART TURNMETHOD OR LOAD INDICATING WASHERS. DO NOT USE CALIBRATED TORQUE WRENCHES. USE A HARDENED WASHER UNDER THE BOLT HEAD OR NUT, WHICHEVER IS ROTATED. THE RE-USE OF FULLY TENSIONED BOLTS IS PROHIBITED.
S7. PROVIDE ALL CLEATS AND DRILL ALL HOLES NECESSARY FOR FIXING STEELTO STEEL OR TIMBER.
S8. FABRICATE STEEL BEAMS AND TRUSSES SPANNING GREATER THAN 5m WITHAN UPWARD PRE CAMBER OF 1/500 SPAN U.N.O.
S9. PREPARE STRUCTURAL STEELWORK TO CLASS 2 AND PAINT WITH ZINC PHOSPHATE PRIMER TO A THICKNESS OF 70 MICROMETRES U.N.O.
S10. HOT DIP GALVANISE ALL EXPOSED EXTERNAL STEELWORK AND ALLSTEELWORK BUILT INTO AN EXTERNAL MASONRY SKIN, IN ACCORDANCEWITH GRADE HDG600 TO AS/NZS 2312.
WITHIN 100m FROM THE NON-SURF COAST OR 1 KM FROM THE SURF COAST, HOT DIP GALVANISE ABOVE IN ACCORDANCE WITH GRADE HDG900 TO AS/NZS 2312.
S11. PROVIDE FIRE PROTECTION TO ALL STEELWORK AS REQUIRED.
S12 ENSURE ALL COLD FORMED SECTIONS CONFORM TO AS/NZS 4600 AND ARE ROLL-FORMED FROM STEEL STRIP, MINIMUM YIELD STRESS 450 MPa,300 g/m2 MINIMUM ZINC COATING MASS U.N.O.

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Project Status	
DESIGNED	DMG
DRAWN	CAD
CHECKED	PDD
APPROVED	PDD

B Jessep
133 Greenpark Road
RD4
Christchurch
Container Shed
Structural Specification

DUNCAN MANAGEMENT GROUP
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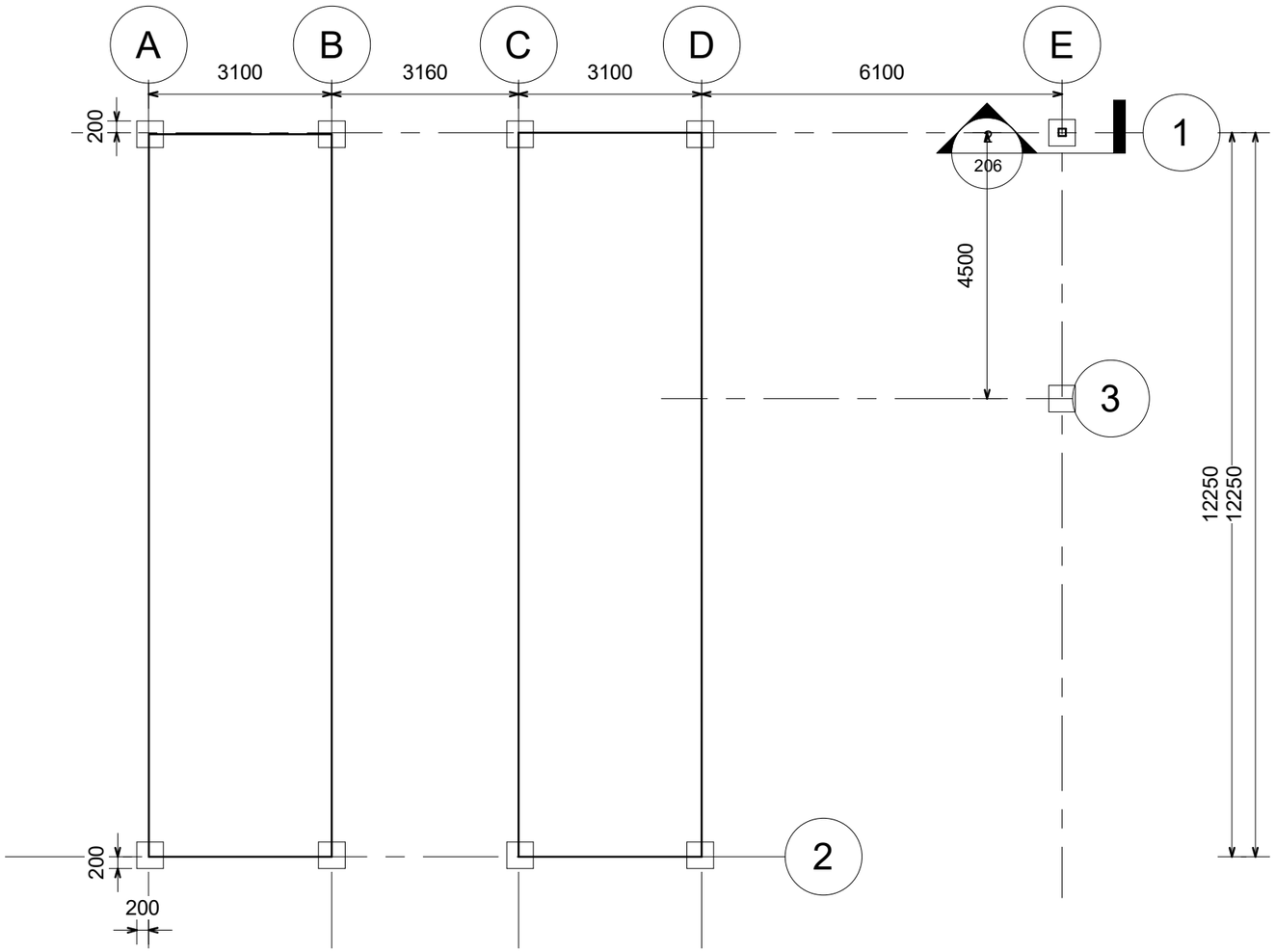
Offices in Australia and New Zealand

ORIGINAL	A3
SCALE	1 : 149
PROJECT NO	190514
SHEET	200
DATE	Issue Date

Revision Schedule		
Revision Number	Revision Description	Revision Date



1 FOOTING PLAN

1 : 100

FIGURED DIMENSIONS TAKE PRECEDENCE.

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Project Status	
DESIGNED	DMG
DRAWN	CAD
CHECKED	PDD
APPROVED	PDD

B Jessep
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Container Shed
Footing Plan

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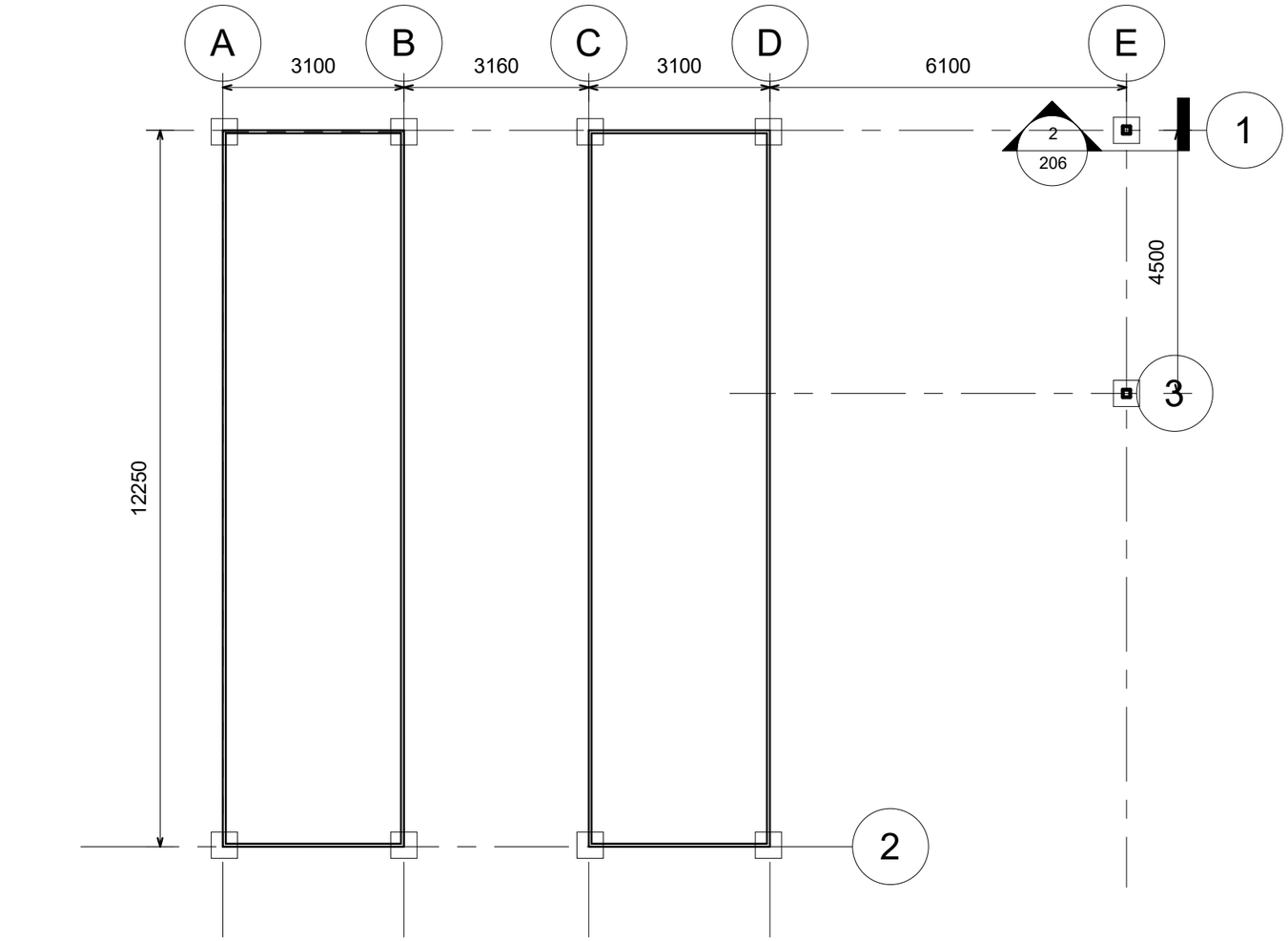
ORIGINAL	A3
SCALE	1 : 100
PROJECT NO	190514
SHEET	201
DATE	Issue Date

Revision Schedule		
Revision Number	Revision Description	Revision Date



1 Floor Plan

1 : 100

FIGURED DIMENSIONS TAKE PRECEDENCE.

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Project Status	
DESIGNED	DMG
DRAWN	CAD
CHECKED	PDD
APPROVED	PDD

B Jessep

133 Greenpark Road
RD4
Christchurch

Container Shed
Floor Plan

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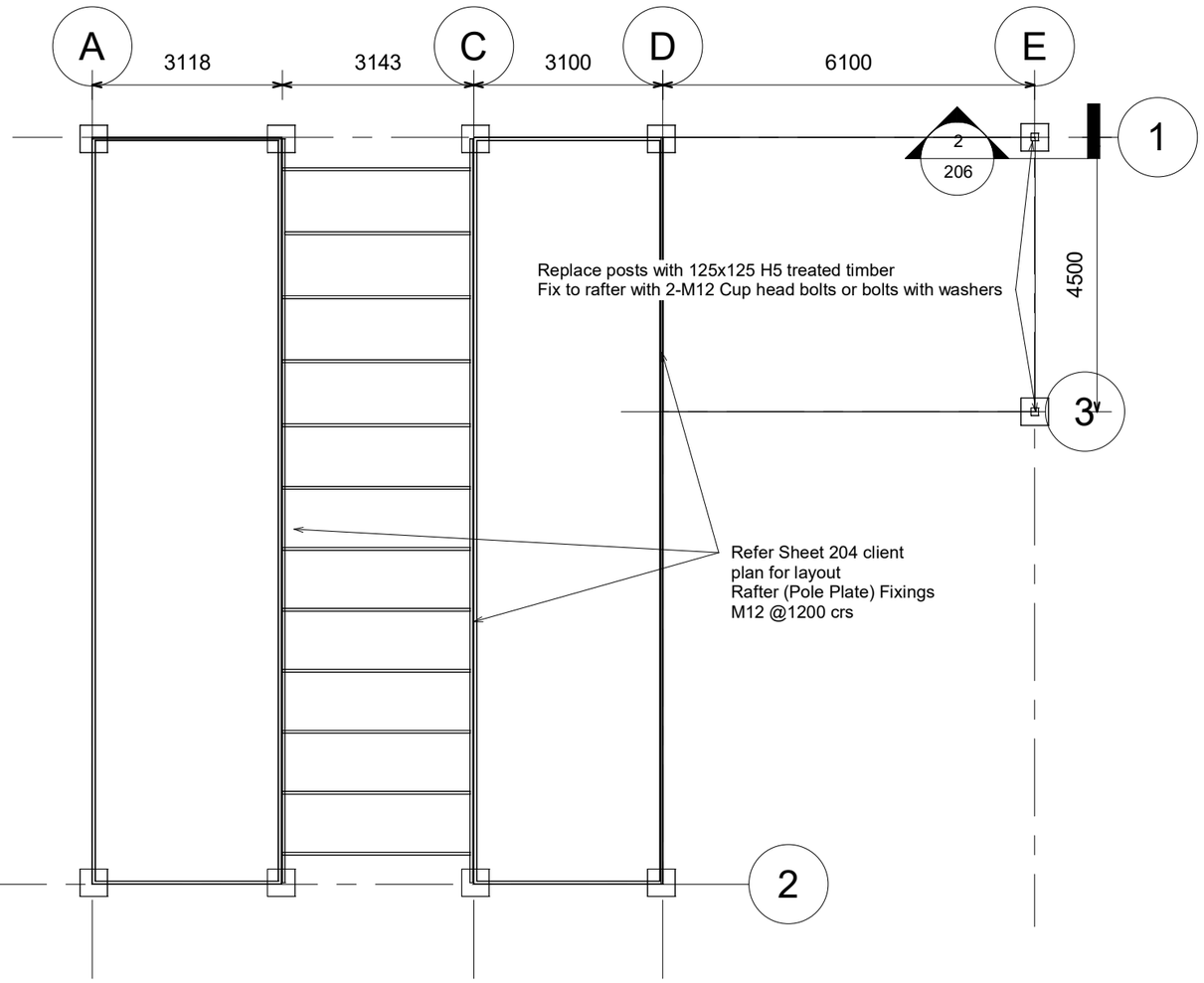
ORIGINAL	A3
SCALE	1 : 100
PROJECT NO	190514
SHEET	202
DATE	Issue Date

Revision Schedule		
Revision Number	Revision Description	Revision Date



1 Roof Plan

1 : 100

FIGURED DIMENSIONS TAKE PRECEDENCE.

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Project Status	
DESIGNED	DMG
DRAWN	CAD
CHECKED	PDD
APPROVED	PDD

B Jessep
133 Greenpark Road
RD4
Christchurch
Container Shed
Roof Plan

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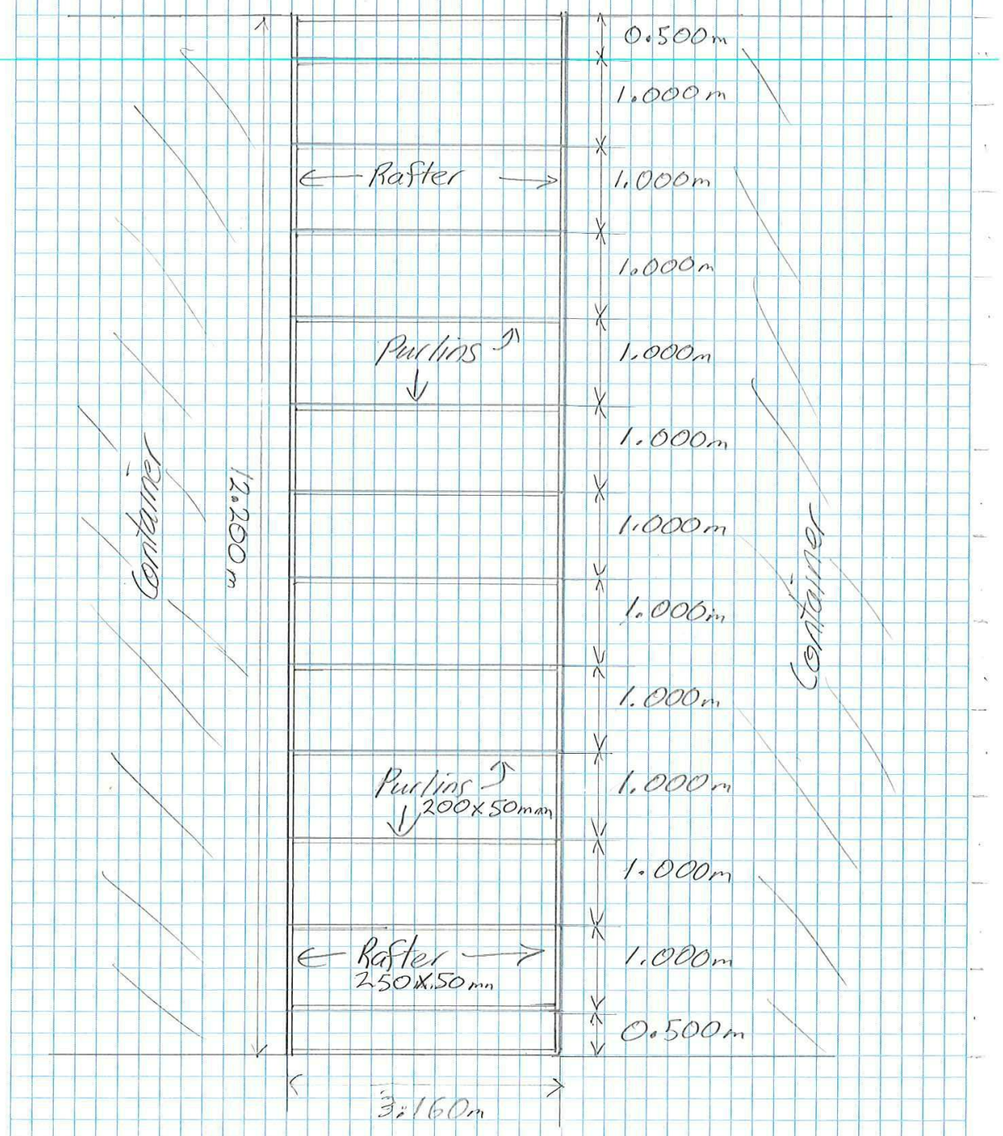
ORIGINAL	A3
SCALE	1 : 100
PROJECT NO	190514
SHEET	203
DATE	Issue Date

Revision Schedule		
Revision Number	Revision Description	Revision Date

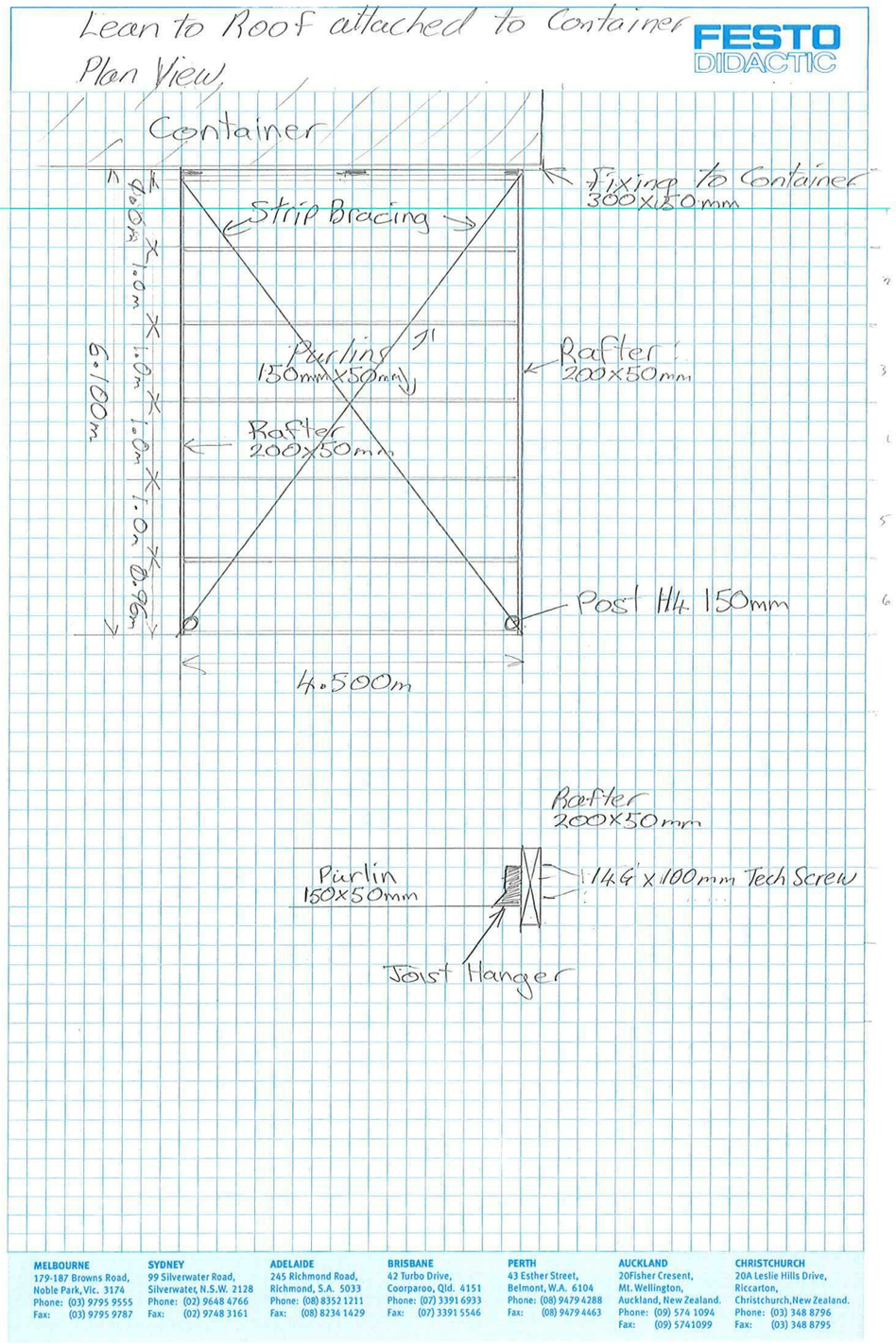
Roof Between Containers

Plan View



Lean to Roof attached to Container

Plan View



MELBOURNE 179-187 Browns Road, Noble Park,Vic. 3174 Phone: (03) 9795 9555 Fax: (03) 9795 9787
SYDNEY 99 Silverwater Road, Silverwater, N.S.W. 2128 Phone: (02) 9648 4766 Fax: (02) 9748 3161
ADELAIDE 245 Richmond Road, Richmond, S.A. 5033 Phone: (08) 8352 1211 Fax: (08) 8234 1429
BRISBANE 42 Turbo Drive, Coorparoo, Qld. 4151 Phone: (07) 3391 6933 Fax: (07) 3391 5546
PERTH 43 Esther Street, Belmont, W.A. 6104 Phone: (08) 9479 4288 Fax: (08) 9479 4463
AUCKLAND 20Fisher Cresent, Mt. Wellington, Auckland, New Zealand. Phone: (09) 574 1094 Fax: (09) 5741099
CHRISTCHURCH 20A Leslie Hills Drive, Riccarton, Christchurch,New Zealand. Phone: (03) 348 8796 Fax: (03) 348 8795

FIGURED DIMENSIONS TAKE PRECEDENCE.

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Project Status	
DESIGNED	DMG
DRAWN	CAD
CHECKED	PDD
APPROVED	PDD

B Jessep
133 Greenpark Road
RD4
Christchurch
Container Shed
Roof Detail

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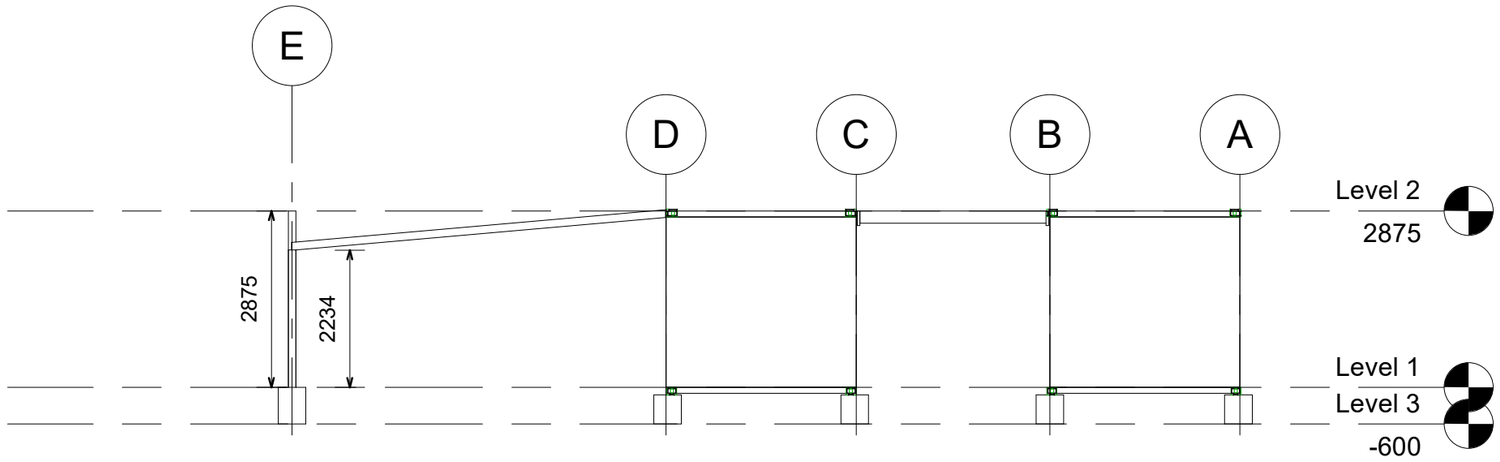
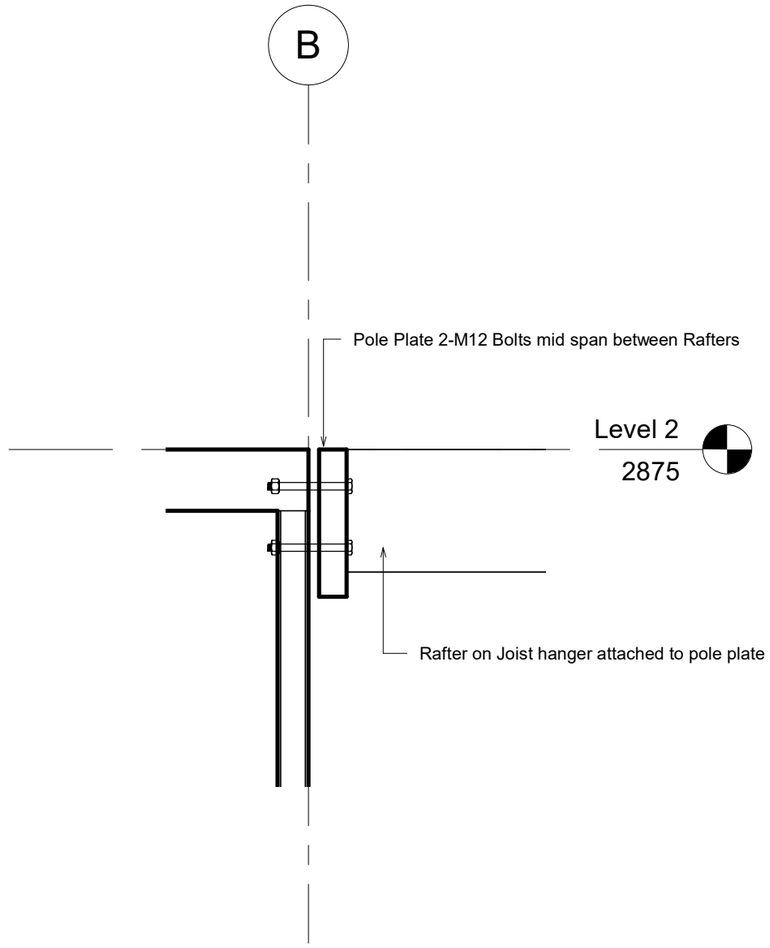
ORIGINAL	A3
SCALE	1 : 50
PROJECT NO	190514
SHEET	204
DATE	Issue Date

Revision Schedule		
Revision Number	Revision Description	Revision Date



1 North

1 : 100



2 Section 2 - Callout 1

1 : 10

FIGURED DIMENSIONS TAKE PRECEDENCE.

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Project Status	
DESIGNED	DMG
DRAWN	CAD
CHECKED	PDD
APPROVED	PDD

B Jessep
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Container Shed
Elevations

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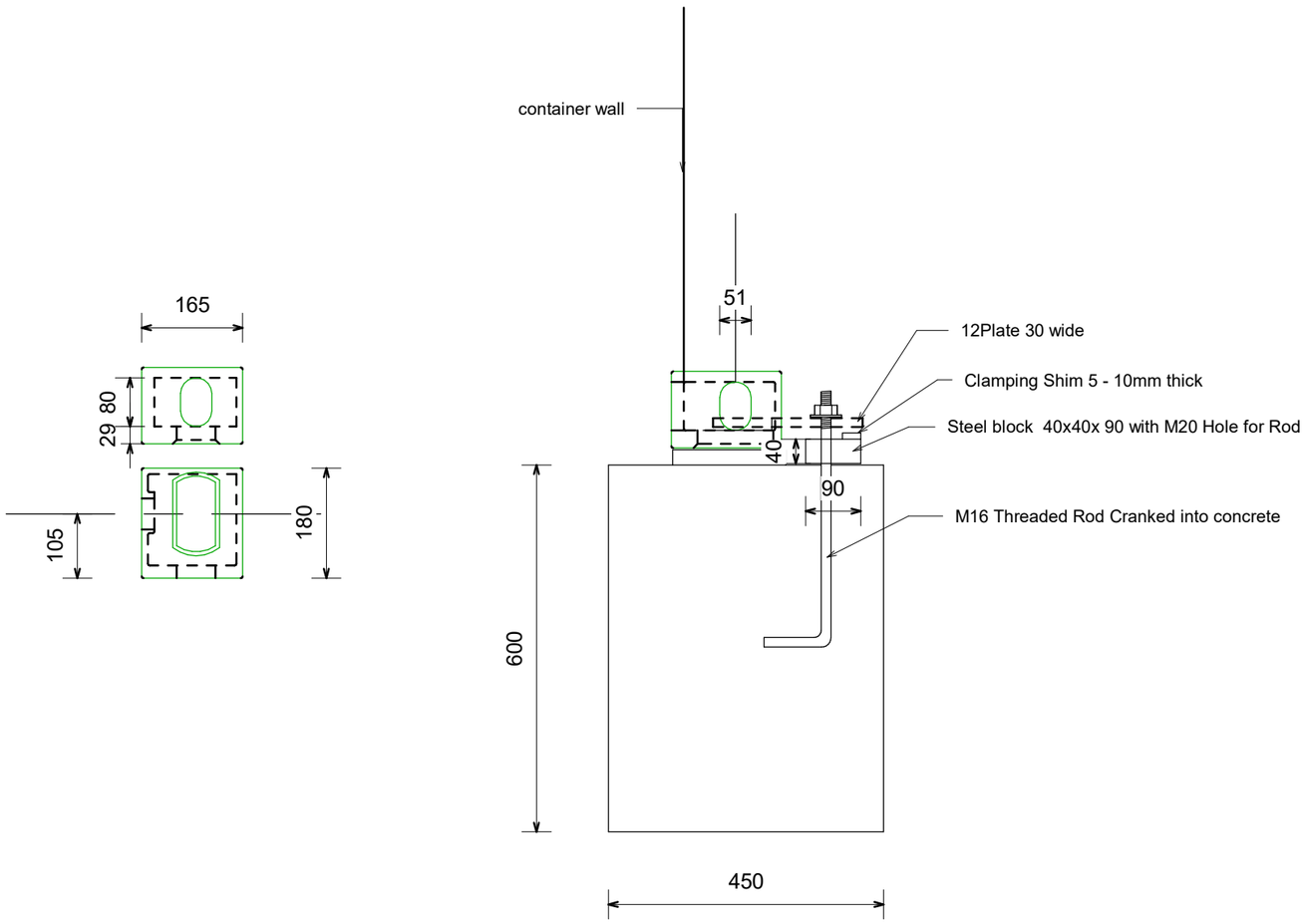
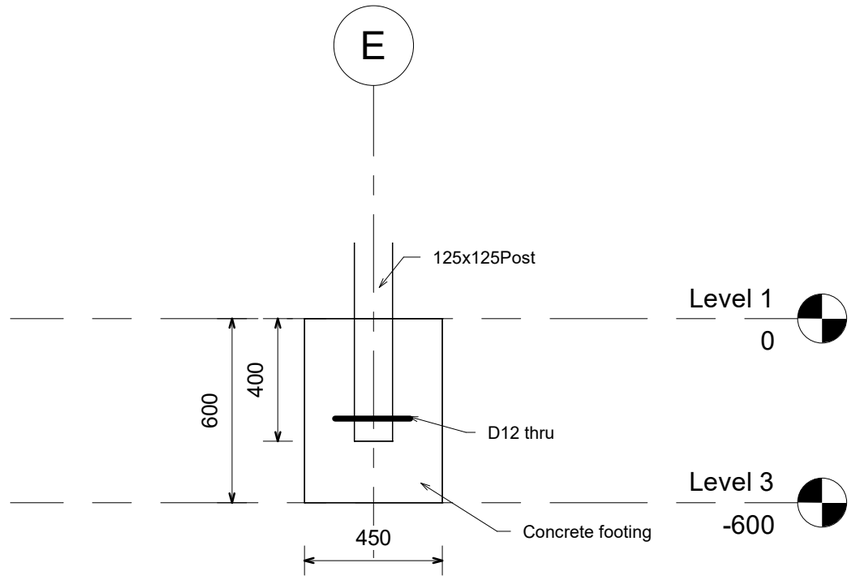
ORIGINAL	A3
SCALE	As indicated
PROJECT NO	190514
SHEET	205
DATE	Issue Date

Revision Schedule		
Revision Number	Revision Description	Revision Date



1 corner detail Tie Down

1 : 10



2 Section 3

1 : 20

FIGURED DIMENSIONS TAKE PRECEDENCE.

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Project Status	
DESIGNED	DMG
DRAWN	CAD
CHECKED	PDD
APPROVED	PDD

B Jessep
133 Greenpark Road
RD4
Christchurch
Container Shed
Details

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project managers

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Offices in Australia and New Zealand

ORIGINAL	A3
SCALE	As indicated
PROJECT NO	190514
SHEET	206
DATE	Issue Date

SKIPSTONE CONSULTING
Peter D Duncan CPEng
Consulting Engineers & Project Managers

21 May 2019 Ref: 190514

Bruce Jessep
133 Greenpark Road
RD4
Christchurch

RE: 133 Greenpark Road RD4 Christchurch – Container Shed and Rood

Skipstone Consulting have been engaged by Bruce Jessep of to provide structural details and tie down for containers and roofs on his property

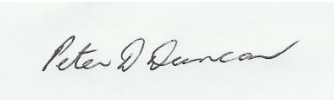
We understand that a “show cause” notice has been issued

I have reviewed the structure and the associated foundation requirements to meet the building code.

The containers can be tied down with clamps which meet the load requirement for the structure and allow for earthquake loading of a minor building structure for storage on site.

Please find attached the related drawings and PS1 for the structural elements described. I can also provide a PS4

Kind Regards



Peter D Duncan CPEng 144221
021 397 008

e. peterd@skipstoneconsulting.co.nz
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1 Port Hills Road, Christchurch, New Zealand 8061

Building Code Clause(s) B1/VM1/VM4

PRODUCER STATEMENT – PS1 – DESIGN

(Guidance on use of Producer Statements (formerly page 2) is available at www.engineeringnz.org)

ISSUED BY: Skipstone Consulting Ltd
(Design Firm)

TO: Bruce Jessep
(Owner/Developer)

TO BE SUPPLIED TO: Selwyn District Council
(Building Consent Authority)

IN RESPECT OF: Container Shed Foundation and Design Check
(Description of Building Work)

AT: 116 McDonald Road
(Address)

Town/City: Christchurch **LOT**..... **DP**..... **SO**.....
(Address)

We have been engaged by the owner/developer referred to above to provide:

Review of Structure and Foundation tie down

(Extent of Engagement)

services in respect of the requirements of Clause(s) B1 of the Building Code for:

☐ All or ■ Part only (as specified in the attachment to this statement), of the proposed building work.

The design carried out by us has been prepared in accordance with:

■ Compliance Documents issued by the Ministry of Business, Innovation & Employment VM1/VM4 or
(verification method/acceptable solution)

☐ Alternative solution as per the attached schedule.....

The proposed building work covered by this producer statement is described on the drawings titled:

Container Shed 1905 and numbered 200-206;
together with the specification, and other documents set out in the schedule attached to this statement.

On behalf of the Design Firm, and subject to:
(i) Site verification of the following design assumptions .....
(ii) All proprietary products meeting their performance specification requirements;

I believe on reasonable grounds that a) the building, if constructed in accordance with the drawings, specifications, and other documents provided or listed in the attached schedule, will comply with the relevant provisions of the Building Code and that b), the persons who have undertaken the design have the necessary competency to do so. I also recommend the following level of construction monitoring/observation:

☐ CM1 ☐ CM2 ■ CM3 ☐ CM4 ☐ CM5 (Engineering Categories) or ☐ as per agreement with owner/developer (Architectural)

I, Peter David Duncan am: ■ CPEng 144221 # ☐ Reg Arch ..... #
(Name of Design Professional)

I am a member of: ■ Engineering New Zealand ☐ NZIA and hold the following qualifications:.....
The Design Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less than $200,000*.
The Design Firm is a member of ACENZ: ☐

SIGNED BY Peter David Duncan (Signature) Digitally signed by Peter D Duncan Date: 2019.05.22 08:44:04 +12'00'
(Name of Design Professional)

ON BEHALF OF Skipstone Consulting Ltd Date 22-5-2019
(Design Firm)

Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of $200,000.*

This form is to accompany **Form 2 of the Building (Forms) Regulations 2004** for the application of a Building Consent.